Lot 2 Hidden Court Kennington

Geotechnical Investigation for Agarius Pty Ltd

> Report 23C 0820-1 November 2023





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GEOTECHNICAL | ENVIRONMENTAL | CONSTRUCTION MATERIALS TESTING Head Office / Mail

13 Alstonvale Court,

East Bendigo VIC 3550

Phone 03 5441 4881

Bendigo Laboratory 13 Alstonvale Court, East Bendigo VIC 3550 Phone 03 5441 4881

Echuca Laboratory Shed 3, 140 Ogilvie Ave, Echuca VIC 3565 Phone 03 5480 0601

Ballarat Laboratory Unit 6, 33 Laidlaw Dr, Delacombe VIC 3356 Phone 03 5303 9993



1 INTRODUCTION

Agarius Pty Ltd commissioned Geotechnical Testing Services (GTS) to conduct a geotechnical investigation for the proposed development at Lot 2 Hidden Court, Kennington.

The investigation has been conducted for the purpose of assessing general subsurface conditions at the site and consequently assigning a Site Classification in accordance with AS2870 - 2011 *Residential Slabs and Footings*.

2 INVESTIGATION

The investigation was conducted on the 23rd of October 2023 using a trailer mounted drill rig to drill 3 boreholes to depths of 1.1 to 1.8 metres within the designated area. The soil profiles and borehole locations are presented at the end of this report.

At the time of this investigation, the type of development proposed is understood by GTS to be a new residential building. If the actual construction varies from this, then changes may be necessary to this classification report.

3 SITE CONDITION

The building envelope of the site is relatively flat and currently vacant. At the time of the investigation, the surface of the site was dry to moist and lacked grass cover. There are no trees within proximity of the proposed development. There was no visual evidence of surface cracking or surface rock. No groundwater seepage was encountered over the investigated depths.

Full details of the soil conditions are presented in the borehole logs.

4 SITE CLASSIFICATION

After allowing due consideration to the site geology, soil conditions, drainage, controlled fill (GTS Report 23C 0198), vegetation including trees and known details of the proposed development, the site has been classified as **Class M-D**.

Class M-D sites have an expected characteristic surface movement (y_s) of 20 to 40mm.

Foundations designed in accordance with this classification are to be subject to the overriding conditions of Section 5.



5 DISCUSSION

Particular attention should be paid to the design of footings as required by AS2870 – 2011.

In addition to the normal founding requirements arising from the above classification, particular conditions at the site dictate that the founding medium and minimum depth below existing surface levels for all footings should be as follows:

• **CONTROLLED FILL:** Silty CLAY, low to medium plasticity, brown, with fine to medium gravel, trace fine to coarse sand, very stiff.

At depth below 0.1 metres in the region of BHs 2 and 3 and not intersected in BH1.

And/or

• **Silty CLAY**, medium plasticity, orange/brown, trace fine gravel, trace fine to coarse sand, very stiff.

At depth below 0.3 metres in the region of BH1, below 0.6 metres in the region of BH3 and at depths below 1.0 metres in the region of BH2.

Or

• **SILTSTONE**, distinctly weathered, pale brown, low to medium strength rock. At depth below 0.8 metres in the region of BH1, at depths below 1.2 metres in the region of BH3 and at depths below 1.35 metres in the region of BH2.

An allowable bearing pressure of 100kPa is available for edge beams, strips and stump footings founded in the controlled fill and natural silty clays, and an allowable bearing pressure of 300kPa is available for edge beams, strips and stump footings founded in the weathered siltstone rock. All foundations should extend a minimum of 100mm into the above foundation material.

If founding on the siltstone rock, bored or screw piers may be considered. Blinding concrete (minimum strength 15MPa) may be used to bring the excavations up to design levels.

The base of all footing excavations must be free of tree roots.

The proposed development should be located a minimum distance of 0.75 x the mature height of all trees. This distance should be increased by 50% for groups or lines of trees. If this distance is impeded, then the size and distance from the development of the tree(s) needs to be taken into account when designing the foundation.

6 IMPORTANT NOTES ABOUT THIS REPORT

 The site classification presented in Section 4 assumes that the current natural drainage and infiltration conditions at the site will not be markedly affected by the proposed site development work. Care should therefore be taken to ensure that surface water is not permitted to collect adjacent to the structure and that significant changes to seasonal soil moisture equilibria do not develop as a result of service trench construction or tree root action.



- Attention is drawn to Appendix B of AS2870 and CSIRO document *BTF 18 Foundation* Maintenance and Footing Performance: A Homeowner's Guide as a guide to maintenance requirement for the proposed structure.
- This is not a comprehensive investigation nor is it economic or practical to determine every subsurface feature on the site. Although this investigation indicates that soil conditions are relatively uniform across the site, it is recommended that the base of all footing excavations be inspected to ensure that the founding medium meets the requirements referenced herein with respect to type and strength of founding materials. If further variations in descriptions in soil types, colour or depths are discovered during construction, this office should be notified immediately so that potential influence on the footings may be assessed.
- The soil colours provided in the borehole logs attached may vary with soil moisture content and individual interpretation, therefore colour alone should not be used to identify these soils.
- Strength characteristics of soils often exhibit a large variation between wet and dry conditions. Soil characteristics of a soil profile are given on the soil conditions at the time of the investigation.
- In the event of significant earthworks being undertaken on the site after this investigation, this report may require an amendment if appropriate.
- If FILL is found during this investigation, it is an indication of what was found during the investigation and it may vary over the site. It may be in the best interest of the buyer/seller to undertake a more detailed investigation, in this instance.

Should you have any further queries concerning these results, please do not hesitate to contact GTS on 03 5441 4881.

Prepared by

Corey Palmer BE (Hons) GradIEAust Graduate Geotechnical Engineer Reviewed by

Hunto

Shane Hampton BE (Hons), MIEAust Principal Geotechnical Engineer





GTS - Bendigo

13 Alstonvale Court East Bendigo VIC 3550 Phone: 03 5441 4881 **Geotechnical Log - Borehole**

1

υтм	:			Drill	Rig	: Honda SCR - Trailer Mount	Job Number	: 23C 08	20-2			
Easting (m)	: 0		Drille	er Supplie	r : Geotechnical Testing Services	Client	: Agariu	s Pty Ltd			
Northing	(m)	: 0		Logg	ged By	: PB	Project	: Site Cla	assification	ı		
Ground E	Elevation :	Not Surve	yed	Revi	ewed By	: CP	Location	: Lot 2 H	lidden Cou	rt, Kennin	gton	
Total Dep	oth :	1.1 m BGL	-	Date	•	: 18/10/2023	Loc Comment	t :				
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						orange brown, trace fine sized gra	vel, trace					
						fine to coarse grained sand, moist	to dry.					
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GEOTECHNICAL TESTING SERVICES
GEOTECHNICAL TESTING SERVICES

GTS - Bendigo

13 Alstonvale Court East Bendigo VIC 3550 Phone: 03 5441 4881 **Geotechnical Log - Borehole**

2

UTM Easting (Northing Ground I Total De	: (m) : (m) : Elevation : pth :	0 0 Not Surve 1.8 m BGL	yed	Drill Drille Logg Revie Date	Rig er Supplie jed By ewed By	: Honda SCR - Trailer Mount r : Geotechnical Testing Services : PB : CP : 18/10/2023	Job Number Client Project Location Loc Comment	: 23C 082 : Agarius : Site Cla : Lot 2 Hi t :	20-2 S Pty Ltd Issification	n rt, Kennin	gton	
Drilling Method	Water	Depth (m)	Soil Origin	Graphic Log	Classification Code	Material Description			Weathering	Moisture	consistency/Density	Testing
100mm SFA		1 -	Controlled Fill Natural		CL-CI	Silty CLAY (CL-CI) : very stiff, low to plasticity, brown, with fine to mediu gravel, trace fine to coarse grained	o medium m sized sand, dry	۱ ۲.		D	VSt	
		-				fine to coarse grained sand, dry.	vel, trace					
		-	Rock		SLT	SILTSTONE: distinctly weathered, medium strength, pale brown, grair	low to ned, dry.		DW	D	LS-MS	
						2 refusal at 1.8 m (Refusal on Siltsto	one Rock)					

GTS - Bendigo

13 Alstonvale Court East Bendigo VIC 3550 Phone: 03 5441 4881 **Geotechnical Log - Borehole**

3

UTM	:			Drill	Rig	: Honda SCR - Trailer Mount	Job Number	: 23C 082	20-2			
Easting (m)	: 0.0		Drille	er Supplie	: Geotechnical Testing Services	Client	: Agarius	Pty Ltd			
Northing	(m)	: 0.0		Load	aed Bv	: PB	Project	: Site Cla	ssificatio	n		
Ground E	Elevation	Not Surve	yed	Revi	iewed By	: CP	Location	: Lot 2 H	idden Cou	rt, Kennin	gton	
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DESCRIPTIVE TERMS BOREHOLE/EXCAVATION LOG

Classification Symbol & Soil Name

Classification of material and its description is based on the Unified Classification System as referenced in AS1726 – 1993 Geotechnical Site Investigations, Appendix A. A summary of the more common terms is included within.

Particle Size Descriptive Terms

Name	Subdivision	Size
Boulders		>200mm
Cobbles		63 – 200mm
Gravel	Coarse	20 – 63mm
	Medium	6 – 20mm
	Fine	2.36 – 6mm
Sand	Coarse	0.6 – 2.36mm
	Medium	200 – 600 micron
	Fine	75 – 200 micron
Silt		2 – 75 micron
Clay		< 2 micron

Consistency of Cohesive Soils

Term	Undrained shear strength, s _u (kPa)	Field Guide
Very Soft (VS)	<12	A finger can be pushed well into the soil with little effort
Soft (S)	12 – 25	A finger can be pushed into the soil to about 25mm depth
Firm (F)	25 – 50	The soil can be indented about 5mm with the thumb
Stiff (St)	50 – 100	The surface of the soil can be indented with the thumb
Very Stiff (VSt)	100 – 200	The surface of the soil can be indented by thumb nail
Hard (H)	>200	The surface of the soil can be marked only with the thumbnail
Friable (F)	-	Crumbles or powders when scraped by thumbnail

Method

s	Auger Screwing	w	Washboring
D	Auger Drilling	N	Natural Exposure
R	Roller/tricone	E	Existing Excavation

Water

*	Not observed
\leq	Observed water level (date shown)

- Observed water inflow
- Observed water outflow
- R Refer to report for details

Structures, Additional Observations

PP	Pocket Penetrometer test (kPa)
DCP	Dynamic Cone Penetrometer test
	(blows/100mm)

Density of Granular Soils

Term	Density Index (%)	
Very Loose (VL)	< 15	
Loose (L)	15 – 35	
Medium Dense (MD)	35 – 65	
Dense (D)	65 - 85	
Very Dense (VD)	> 85	

Minor Components

Term	Field Guide	Proportion of Minor Component In:
Trace of	Presence just detectable by feel or eye	Coarse grained soils: <5% Fine grained soils: <15%
Some	Presence easily detectable by feel or eye	Coarse grained soils: 5-12% Fine grained soils: 15-30%

Moisture Condition

Dry (D)	Looks & feels dry. Cohesive soils are usually hard, powdery or friable. Granular soils run freely through the hand.
Moist (M)	Soil feels cool and darkened in colour. Cohesive

- soils can be moulded. Granular soils tend to cohere. Free water does not form.
- Wet (W) As for moist, but with free water forming on hands when remoulded.

Support

В	Blade/bucket	*	Nil
С	Coring	С	Casing
н	Hammer Drill	м	Mud/polvmer

Notes, Samples, Tests

U63 Undisturbed sample, 63mm diameter

D Disturbed sample
N* Standard Penetration Test, (*) Sample

Figure = results

Surface

	Known boundary
	Probably boundary
-?-?-?-?-?-	Possible boundary